

Analysis of crash frequency in motorway tunnels based on a correlated random-parameters approach

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Abstract

The paper provides an analysis of the frequency of total accidents (accidents involving material damage, physical injuries and fatalities), which occurred in 226 unidirectional motorway tunnels over a four-year monitoring period, based on unrelated and correlated random-parameter models. The so-called random-intercept model, in which only the regression intercept is assumed to be random, was also developed a priori for recording the random-effects (temporal correlations among accidents occurring in the same tunnel in different years). The independent variables were: tunnel length (L), annual average daily traffic (AADT) per lane, percentage of trucks ($\%Tr$), presence of a sidewalk (SW), longitudinal slope (LS), and mechanical ventilation (MV). The comparison among the aforementioned three models showed that the correlated random-parameters model, which takes into account the cross correlation among the random-parameters, provided a better goodness-of-fit than the corresponding uncorrelated random-parameter and intercept-random models. This means that more precise estimations of accidents can be obtained when the random-parameters are assumed to be correlated in statistical analysis. The developed model also offers additional insights into showing how different combinations of parameters affect tunnel safety. In particular, through the correlation coefficient matrix of random-parameters, we found that the non-constant longitudinal slope (LS) alleviates the effect of the annual average daily traffic (AADT) per lane on increasing crash frequency. In addition, the presence of the mechanical ventilation (MV) in tunnels makes less significant the influence of AADT per lane on increasing crash frequency, too. The knowledge of these correlations may be useful for future applications, for example, for road engineers in designing tunnel. The model proposed can also be used by Tunnel Management Agencies (TMAs) for estimating possible variations in accident frequency in a specific tunnel due to modifications of traffic control systems.

Keywords: Crash frequency, Correlated random-parameters model, Poisson distribution, Binomial Negative distribution, Road tunnels.

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1. Introduction

Roads accidents throughout the world, according to the World Health Organization (WHO, 2015), cost the lives of 1.25 million people every year. Moreover, these accidents are the main cause of death among young people aged between 15 and 29 years (over 300,000 deaths). In the 28 countries of the European Union (EU28, 2016) the number of road accident fatalities was 25,720 in 2016. With regard to Italy, the Italian National Institute for Statistics (ISTAT, 2016) reports that in the same year 2016 road accidents caused 3,283 deaths and 249,175 injuries.

Road accidents cause so much pain and suffering which cannot be measured or quantified, as well as high social costs in terms of medical expenses and lost productivity. All this costs governments approximately 3% of the Gross Domestic Product (WHO, 2015).

The high human and economic toll that road accidents continue to inflict on society is no longer sustainable. Moreover, it is to be said that the sustainable development goal fixed by the WHO, which included the target of a 50% reduction in road traffic deaths and injuries by 2020, does not appear achievable.

However, so much effort has unquestionably been exerted over time to improve road safety that nowadays we can observe a drastic reduction in serious accidents compared to those of previous years. This is due more especially to: (i) the enforcement of road safety policies and road user behaviour (e.g., reducing speed, increasing helmet and seat-belt use, reducing drink- and drug-driving); (ii) safer vehicles (e.g., better

standards protecting vehicle occupants, electronic stability control, new features on board to avoid collisions with pedestrians); (iii) safer roads (higher levels of infrastructure design and maintenance, also with reference to non-motorized road users). However, in contrast with other main transportation systems (such as air, water, and railway transport) where human behaviour in driving is widely controlled by safety protocols and sophisticated electronic equipment, the road transport system still remains quite dangerous. This might be attributed to different physical and mental capabilities of drivers, dissimilar perceptions of risk level, different reactions to external stimuli. In addition, distracted driving and/or talking on mobile phone while driving, are serious and ever growing threats to road safety.

The recent introduction of autonomous driving vehicles, which can potentially remove human error, might eventually lead to significant advantages in road safety. However, the large-scale spread of these vehicles is nowadays still questionable due to both the occurrence of accidents that caused the death of some drivers during tests and the interaction with other vehicles when variations in traffic flow and road characteristics are expected, as well as driving in poor weather conditions.

While the success of the aforementioned efforts in reducing road accidents and mitigating their impact cannot be denied, research based on the statistical analysis of accident data-bases is also recognized as being a useful tool for contributing in the sustainable development of road-safety policies towards saving lives and reducing the severity of injuries. However existing data-bases, which usually extract data from police accident reports do not contain all the information necessary for defining human behaviour accurately, traffic and road characteristics, weather, and in what way drivers react to continuous variations in environmental conditions while driving. In other words other elements remain unobserved by analysis. In the light of this limit of traditional accident data-base, more advanced statistical methods have been developed in order to address this relevant research question known to analysts as “unobserved heterogeneity”.

Characteristics of horizontal and vertical alignment may cause unobserved heterogeneity through observations due, for instance, to different reactions of drivers to road geometry.

Traffic also may be responsible for unobserved heterogeneity through observations attributable, for example, to variations in driving style of road users in response to traffic modifications.

Vehicle characteristics may be another example of unobserved heterogeneity caused by size, weight, structure of vehicle, as well as by vehicle speed differences.

Moreover, human stimuli linked, for instance, to gender, age, height, weight, educational level, which are generally unavailable for analysis, may also cause unobserved heterogeneity through observations.

Finally, when some of the most important explanatory variables affecting accidents are omitted, because they have not been observed (and therefore not reported in the data-base), additional unobserved heterogeneity is introduced.

Several statistical methods have been applied over the years for road accident analysis in order to develop predictive models and evaluate the significance of variables affecting crashes. In the last decade, random-parameters models have been more especially considered. These models try to record the aforementioned unobserved heterogeneity (unobserved factors that may vary through accident observations) by making it possible that the regression model parameters are random, in contrast with the assumption that parameters are fixed through observations (traditional statistical approach).

A special case in the class of random-parameters models, is the so-called random-intercept model in which only the regression intercept is assumed to be random. Therefore, this type of model records only an unobserved specific heterogeneity which is unrelated to explanatory variables, for example the so-called unobserved temporal heterogeneity. This unobserved heterogeneity, varying over time on a specific segment of a road, might include weather-related factors and the reaction of drivers in response to weather variations that cannot be observed by the analyst. In keeping with the international literature, this model is known to be equivalent to the random-effects one. Random-effects models are necessary when temporal correlations (data collected on the same road section over successive time periods) or spatial correlations (data collected from the same geographic area) are thought to be present.

Generally speaking, it is to be said that in the field of accident analysis it is assumed that the effects of unobserved heterogeneity are independent one from another. In other words, it is often assumed that there is no correlation among parameters that are found to be random in the regression model developed. However, this might not be true, and ignoring the interaction among random-parameters might lead to less precise accident predictions. This is an issue that researchers have little investigated over time, and so very few studies are available in the literature (some references are reported in the section background of the present paper) and indicate a gap in our knowledge. Accounting for correlation among random-parameters can be

103 achieved by means of applying the so-called correlated random-parameters approach, which is the main
104 scope of the present paper.

105 Various studies exist in the literature on unobserved heterogeneity associated with accidents occurring on
106 open roads, while the occurrence of accidents inside road tunnels have been much less investigated. Driving
107 behaviour in road tunnels is different when compared to that on open roads. Driving in tunnels may cause
108 anxiety because tunnels are dark, narrow and monotonous. Moreover, drivers may also be frightened of
109 hitting other vehicles and the tunnel wall, or of dangerous scenarios such as fires. Moreover, the sunshine
110 reflected from the tunnel portal might cause ocular blinding of drivers before entering the tunnel. Therefore,
111 there is evidence that accidents in road tunnels need to be better examined; which is another intent of this
112 paper. In this respect it is to be mentioned that, according to an Italian study (Caliendo and De Guglielmo,
113 2012), the average rate of severe accidents (accidents involving only injuries and fatalities) was computed to
114 be of 12 severe accidents / 10^8 veh./km in Italian motorway tunnels against 9 severe accidents / 10^8 veh./km on
115 the corresponding motorways containing the tunnels investigated. Also Zhuang-lin et al. (2009) showed that
116 the severity of injuries is higher in Chinese freeway tunnels than on open roads. Yeung et al. (2013) revealed,
117 on the other hand, that traffic accident rates in Singapore expressways tunnels were higher in the transition
118 zones rather than in the interior zones of tunnels. The small number of studies on the parameters that
119 influence traffic safety in road tunnels represents, according to the authors of this article, a further lack of
120 knowledge.

121 As a result of the above considerations, there are at least three main reasons for justifying this paper. The
122 first is motivated by the need to investigate in greater depth on crash frequency (accidents per year) without
123 ignoring possible interactions among random-parameters. In this respect, predictive models based on a
124 correlated random-parameters approach should be developed so that new insights might be provided for
125 increasing our knowledge. The second, given the lacuna of studies on accidents occurring in road tunnels, is
126 to have a better understanding of the relationships between crash frequency in road tunnels and geometric-
127 traffic variables; as well as providing additional insights for road engineers in designing tunnel. In fact there
128 is a priori reason for believing that the effects of unobserved heterogeneity, associated with accidents in road
129 tunnels, might be different from those of unobserved heterogeneity on open roads. Finally, since the
130 statistical methodology generally used in accident analysis is based on the Negative Binomial (NB)
131 distribution, it is interesting to examine in greater depth whether the NB model may converge to the Poisson
132 model in presence of random-parameters assumed to be unrelated or correlated.

133 Therefore, the present paper may make a significant contribution to the state-of-the-art by showing the
134 effects of combined parameters on the occurrence of accidents, and by making proposals in the design of
135 tunnel geometry in order to improve safety.

136 The paper is organized as follows: the next section deals with the statistical methodology applied. Then the
137 data set used and the process of preparing it in statistical analysis are described. Subsequently the results are
138 presented and discussed and appropriate comparisons are made among the predictive models developed.
139 Finally comments for practical applications, conclusions and addresses for additional research are made.

140

141 2. Background

142 Considerable literature exists on the merits and demerits both of traditional statistical models and new ones.
143 Some international references can be found more especially in Washington et al. (2011), Mannering and
144 Bhat (2014), Mannering et al. (2016); as well as with reference to Italian studies in Caliendo et al. (2007,
145 2013), Caliendo and Guida (2014). However, in the last decade, the potentiality of random-parameter models
146 compared to the fixed-parameters one has been particularly investigated. Random-parameter models are
147 considered in Milton et al. (2008), Anastasopoulos and Mannering (2009), Anastasopoulos and Mannering
148 (2011), Caliendo et al. (2016), Caliendo and De Guglielmo (2017). It is to be remembered that the model in
149 which only the regression intercept is random, in the literature, is known to be equivalent to the random-
150 effects model (see Greene (2007); and Hilbe (2007) for in-depth knowledge).

151 However, it is to be stressed that in the aforementioned random-parameters models the distribution of
152 random-parameters is assumed to be independent. In other words, the potential effects of the cross
153 correlation among random-parameters cannot be captured. From the viewpoint of statistical modelling,
154 according to Conway and Kniesner (1991) ignoring the correlations among random-parameters might lead to
155 biased estimations. As a result, researchers have recently been trying to expand methodological frontiers by
156 taking into account the issue mentioned above.

157 Yu et al. (2015), for example, adopted a correlated random-parameter approach in order to investigate the
 158 effects of weather conditions on the risk of crashes on freeways. In their study they found that the correlated
 159 random-parameters Tobit model was statistically superior to the corresponding unrelated-parameters model.
 160 By investigating the crash frequency of many cities, Coruh et al. (2015) showed that the correlated random-
 161 parameters Negative Binomial (NB) model is more appropriate than the unrelated random-parameters NB
 162 model. In analysing crash frequency in the tunnels of China, Hou et al. (2018) also proved that the correlated
 163 random-parameters NB model provided a better goodness-of-fit compared to the unrelated one.

164 It is to be stressed that the present paper is different from our previous studies mentioned above: (i) for
 165 introducing two additional independent variables such as the longitudinal slope and mechanical ventilation;
 166 (ii) for non-assigning a priori a functional form for describing the dependence of the expected number of
 167 accidents on traffic flow (i.e., we did not use the Hauer and Bamfo approach (1997)); (iii) for taking into
 168 account possible correlations among the variables investigated.

169 However, it is evident that very few studies are hitherto available in the literature concerning the correlated
 170 random-parameters models. Moreover, only one study deals with accidents occurring in tunnels. In addition,
 171 it is to be stressed that the results mentioned above, which are based only on Chinese tunnels, might not be
 172 directly transferable to other geographic areas where traffic, tunnel geometry, and driving style are different.
 173 This may be the case of Italy. Therefore, given the lacuna of studies on the aforementioned issues and doubts
 174 on transferability of results to other countries, the potential of considering combined random-parameters
 175 should be investigated in greater depth, and the frontier methodological should be expanded in applying this
 176 statistical approach, which appears to be innovative in the field of accident analysis, also to Italian tunnels.
 177 This might make additional knowledge possible and help in tunnel design.

179 3. Methodology

180 The statistical methodology which is generally applied in crash analysis is based on the assumption that the
 181 fluctuation of accident counts, say Y_i , which occur on a road section i (or within a tunnel) during the
 182 observational time interval, is a Negative Binomial (NB) random variable. Unlike the Poisson model, the NB
 183 model allows for the variance of accident counts to be greater than the mean, provided by the so-called
 184 “overdispersion parameter” (α). When the “overdispersion parameter” α of the NB model converges to zero
 185 (i.e., the “inverse dispersion” parameter $\varphi = 1/\alpha$ diverges to infinity), it suggests that the corresponding
 186 Poisson model is still statistically appropriate.

187 A regression model of the expected number of accidents is defined by typically using the log-linear function,

$$188 \lambda_i = \exp(\mathbf{X}_i^T \boldsymbol{\beta}) \quad (1)$$

189 where $\boldsymbol{\beta}$ is a vector of fixed, even if unknown, coefficients and \mathbf{X} is a vector of k covariates.

190 In order to take into account unobserved heterogeneity which may vary across road sections (or tunnels), a
 191 suitable approach is, however, considering the $\boldsymbol{\beta}$ parameters as random variables, by assuming:

$$192 \boldsymbol{\beta}_i = \boldsymbol{\beta} + \delta_i \quad (2)$$

193 where δ_i is a random variable with some probability density function $f(\delta)$, for example a normal variable with
 194 zero mean and constant standard deviation. If the variance of the chosen distribution is not significantly
 195 different from zero, it means that a conventional fixed-parameters model is still statistically appropriate.

196 The aforementioned approach assumes that the possible unobserved heterogeneity can be captured by
 197 considering that the random-parameters are independent (i.e., there is no correlation among random-
 198 parameters in the model). Indeed, there might be correlation among random-parameters so that a correlated
 199 approach is needed.

200 In order to take into account the potential correlation among the random parameters, $\boldsymbol{\beta}_i$ is assumed to follow
 201 a multivariate normal distribution written as follows:

$$202 \boldsymbol{\beta}_i = \mathbf{b} + \mathbf{C}\boldsymbol{\omega}_i \quad (3)$$

203 with \mathbf{b} and \mathbf{C} :

209

$$\mathbf{b} = \begin{bmatrix} \beta_1 \\ \beta_2 \\ \dots \\ \beta_{j-1} \\ \beta_j \end{bmatrix} \text{ and } \mathbf{C} = \begin{bmatrix} (\sigma_1)^2 & \sigma_{1,2} & \dots & \sigma_{1,j-1} & \sigma_{1,j} \\ \sigma_{2,1} & (\sigma_2)^2 & \dots & \sigma_{2,j-2} & \sigma_{2,j} \\ \dots & \dots & \dots & \dots & \dots \\ \sigma_{j-1,1} & \sigma_{j-1,2} & \dots & (\sigma_{j-1})^2 & \sigma_{j-1,j} \\ \sigma_{j,1} & \sigma_{j,2} & \dots & \sigma_{j,j-1} & (\sigma_j)^2 \end{bmatrix} \quad (4)$$

210

211 In equation (3) β_i is own the random-parameters vector for observation i , \mathbf{b} is the mean vector, \mathbf{C} is the
 212 variance-covariance matrix (\mathbf{C} engenders correlation among the elements of the parameters vector β_i), j is
 213 the number of random parameters, and ω_i is the randomly and independently distributed uncorrelated vector
 214 term. Obviously in the unrelated random-parameters models, the off-diagonal elements in the variance-
 215 covariance matrix are equal to zero.

216 The model parameters were estimated by a maximization procedure of the likelihood function. It is to be
 217 noted, however, that traditional procedures based on numerical maximization of log-likelihood are, in most
 218 cases, unfeasible when the number of unknown parameters is high. Thus, Green (2007) developed an
 219 efficient simulation procedure, based on the Halton draws (see in detail Halton (1960)), which allows
 220 estimating the model parameters even in the presence of a high number of random parameters. In this
 221 respect, an approach based on 2000 Halton draws is used in the present paper in order to obtain an efficient
 222 convergence for numerical integration in estimation.

223

224 4. Data description

225 A 4-year monitoring period, extending from 2006 to 2009, was considered for Italian motorway tunnels. A
 226 number of 226 unidirectional tunnels with two lanes were in particular investigated. The data-base consists
 227 of 1930 total accidents (i.e., accidents involving material damage, injuries and fatalities). The number of total
 228 accidents observed for each year was as follows: 664 (2006), 545(2007), 384 (2008), 337 (2009). The width
 229 of each lane is 3.75 m, the presence of a sidewalk (with a width approximately of 1.0 m) was recorded in 143
 230 tunnels, while the emergency lane was always absent in all tunnels considered. In this respect, it is to be said
 231 that an emergency lane is not generally present in the existing Italian tunnels that were designed and built
 232 several years ago, in contrast with the corresponding open road sections. This is attributable to the decision
 233 to reduce tunnel construction costs. However, nowadays for the design of new motorway tunnels, according
 234 to the more recent Italian standards (MIT, 2002), the emergency lane must be introduced also in tunnels (as
 235 on open roads) in order to achieve higher safety levels.

236 The length of the tunnels investigated varies between 0.39 km and 3.25 km. The longitudinal slope is
 237 constant in 144 tunnels (93 with ascending gradients, and 51 with descending gradients), while varies in the
 238 remaining 82 tunnels with positive and negative gradients. The mechanical ventilation system, which is
 239 longitudinal with jet-fans, is present in 55 tunnels and absent in the remaining 171 tunnels.

240 Unidirectional traffic flow in each tunnel, which is expressed in terms of the annual average daily traffic
 241 (AADT), was found to be between 2250 and 20,380 vehicles/day per lane. The percentage of trucks ranged
 242 from 15% to 31%. Summary statistic of tunnel length, AADT per lane and percentage of trucks are given in
 243 Table 1.

244

245

Table 1 Summary statistic of variables

Variables	Mean	Mode	Standard deviation	Minimum	Maximum
Length [km]	1.20	0.53	0.64	0.39	3.25
AADT /10,000 per lane [veh./day]	0.7620	0.3063	0.4224	0.2250	2.0380
Percentage of trucks [%]	21%	16%	4%	15%	31%

246

247 It is to be mentioned that the aforementioned data-base of accidents is based on official data available at the
 248 Italian Ministry of Infrastructures and Transport. As far as the authors are aware, an update of these data (i.e.
 249 accidents occurring in tunnels in more recent years) was not yet available for a significant number of tunnels
 250 at the time of the writing of this paper. However, the main intent of this paper is not to show the trend of
 251 accidents occurring in tunnels over time (even if according to the Italian National Institute of Statistics
 252 (ISTAT, 2016) accidents continue to decrease on open roads; the same thing which might be expected also in

253 tunnels); but to individualise a model which was statistically superior for taking into account the unobserved
 254 heterogeneity in accident data, and which provided more precise predictive estimations of accidents in
 255 tunnels. Therefore, the authors are confident that the following analysis, which is based on the
 256 aforementioned data-base, is appropriate in relation to the designated objective.

257

258 5. Statistical analysis of data

259 *Set of variables*

260 The dependent variable is assumed to be the frequency of total accidents (number of total accidents per
 261 year). The independent variables are: length (L), average annual daily traffic ($AADT$) per lane, percentage of
 262 trucks ($Tr\%$), sidewalk (SW), longitudinal slope (LS), and mechanical ventilation (MV). The variables SW ,
 263 LS , and MV are assumed to be dummy variables: $SW = 1$ if there is a sidewalk, 0 otherwise; $LS = 1$ if the
 264 longitudinal slope of tunnel varies with ascending and descending gradient, 0 otherwise; $MV = 1$ if there is
 265 the mechanical ventilation, 0 otherwise.

266 Since the intent of this paper is also to record (within the aforementioned monitoring period) the year effect
 267 on the occurrence of total accidents in tunnels, the dummy variables year 2007, year 2008, and year 2009
 268 were also considered, by assuming 2006 as year of reference.

269 *Procedure for choosing the statistically more significant variables*

270 Since some of the potential covariates could actually have very little or even no effect on accident counts, a
 271 procedure based on the Likelihood Ratio Test (LRT) was used in order to decide which subset of the full set
 272 of potentially explanatory variables should be included in the regression model. The likelihood-ratio statistic
 273

$$274 \Lambda = 2 \left[l(\hat{\beta}, \hat{\phi}) - l(\hat{\beta}', \beta_k = 0, \hat{\phi}) \right] \quad (5)$$

275 where $l(\hat{\beta}, \hat{\phi})$ is the log-likelihood of the regression model containing all the covariates, and
 276 $l(\hat{\beta}', \beta_k = 0, \hat{\phi})$ is the log-likelihood of the regression model with the k -th covariate out, is asymptotically
 277 distributed as a chi-squared distribution (χ^2) with 1 degree of freedom. Variables were assumed to be
 278 significant in each model at a significance level of much less than 0.05 (this reflects at least a 95%
 279 confidence).

280 The aforementioned LRT statistic was also used between two competing models in order to test the statistical
 281 superiority of the target model. In this case, the χ^2 statistic is with the degrees of freedom depending on the
 282 difference between two competing models. For example, in the present paper the LRT was also used to
 283 decide if the unrelated random-parameters model were statistically superior to the corresponding random-
 284 intercept model; in this case the number of degrees of freedom (dof) is equal to the number of parameters
 285 that are effectively random.

286 The LRT was also applied for understanding if the correlated random-parameters model were statistically
 287 better than the unrelated random-parameters one. In the comparison between these two models the number of
 288 degrees of freedom is $k = m(m - 1)/2$, where m is the number of the random parameters that are
 289 effectively correlated.

290 However, first of all, on the basis of the aforementioned data-base, a random-intercept Negative Binomial
 291 (RINB) model was developed. Moreover, a random-parameters Negative Binomial model in which both the
 292 intercept and parameters are allowed to vary randomly (RPINB) was also developed. It was found that the
 293 “inverse dispersion” parameter (ϕ) both of the RINB model and RPINB one diverged respectively to
 294 infinity (i.e., the over-dispersion parameter α converged to zero), thus indicating that the outcomes of RINB
 295 and RPINB models converged to the corresponding Poisson models. For these reasons only the Poisson
 296 model with the random intercept, as well as the Poisson model where both the intercept and regression
 297 parameters are allowed to vary randomly, are more especially commented in this paper.

298 *Panel data*

299 The statistical analysis presented in this paper involves the number of total accidents reported for each tunnel
 300 in each year from 2006 to 2009. It is to be remembered that the number of accidents occurring in the same
 301 tunnel in different years are not stochastically independent, but that they share at least the effects of
 302 modifications and/or improvements introduced in that specific road tunnel over time. Thereby it is logic to
 303 consider that they form a cluster of observations sharing common, even if unknown, random-effects.
 304 Random-effects models analyse data by clusters, thus they may take into account the possible temporal
 305 correlation among observations relating to the same tunnel in different years. It is known that random-effects

306 are the same within each panel of cluster, but they differ across clusters. However, the regression parameters
 307 are always the same within and across clusters; in other words the effect-random models do not consider that
 308 the regression parameters can vary across clusters. Therefore, other types of models have been introduced as
 309 the random-parameters models. In order to resolve the issue, we used the LIMDEP statistical package
 310 (Greene, 2007) in which a panel-data vector is contained for taking into account also the fact that the
 311 random-parameters can be fixed within clusters and vary between them.

312

313 6. Analysis of results

314 In order to analyse the effects of the aforementioned set of independent variables and of year covariate on
 315 total accident counts observed in unidirectional motorway tunnels, the following three predictive models
 316 were developed.

317 *Random-intercept Poisson (RIP) model*

318 The results of the random-intercept Poisson (RIP) model (which is equivalent to the random-effects Poisson
 319 (REP) model) are reported in Table 2. Table 2 confirms, coherently with the previous studies of the authors
 320 of this paper, that each variable has the sign expected: the frequency of total accidents is positively
 321 associated in a non-linear way with the tunnel length (L), the annual average daily traffic ($AADT$) per lane,
 322 and the percentage of trucks ($\%Tr$). Possible explanations may be given. By increasing L higher crash
 323 frequency, attributable to the drivers' diminishing concentration, is expected. As $AADT$ per lane and $\%Tr$
 324 increase, the frequency of accidents is also expected to increase because in order to maintain the desired
 325 speeds, the lane changing and overtaking movements, more especially of cars, become more numerous.

326 A reduction in crash frequency over years from 2006 to 2009 is also confirmed, which agrees with accident
 327 data observed. This decrease might be attributable to the positive effects of more stringent road-safety
 328 policies over time and/or to the implementation of certain safety measures in tunnels after the date of the
 329 coming into force in Italy (October 2006) of the European Directive 2004/54/EC (European Parliament and
 330 Council, 2004).

331 All variables are statistically significant at a level of much less than 0.05, except: the presence of sidewalk
 332 (SW), the longitudinal slope (LS), and the mechanical ventilation (MV) (χ^2 with 1 dof < 3.84).

333

334 **Table 2** Estimations results of the random-intercept Poisson (RIP) model (or random-effects Poisson (REP) model).

335

Random Intercept Poisson (RIP) Model			
Variables	Point Estimate	Standard Error	LRT Statistic
Fixed Parameters			
Log of length [km]	1.28366	0.15622	28.84
Log of AADT per lane /10,000	2.85643	0.12685	159.15
Log of percentage of trucks/100	1.82361	0.34981	11.22
SW (Sidewalk: 1 if present; 0 otherwise)	-0.08198	0.07290	0.53
LS (Longitudinal slope: 1 if not constant; 0 otherwise)	0.11396	0.04823	2.13
MV (Mechanical ventilation: 1 if present; 0 otherwise)	0.09152	0.07692	0.60
Y2007	-0.21811	0.07852	14.29
Y2008	-0.53578	0.07039	72.09
Y2009	-0.61406	0.07363	80.52
Random Parameters			
Constant	2.50738	0.26053	-
<i>standard deviation of parameter distribution: 0.36861</i>			
<i>standard error of standard deviation: 0.02365</i>			
Other statistical information			
Number of observations		904	
Log likelihood function $LL(\beta_{RIP Model})$		-1454.20814	

336 *Unrelated random-parameters Poisson (URPP) model*337 Table 3 shows the results of the unrelated random-parameters Poisson (URPP) model in which both intercept
338 and parameters are assumed to vary randomly.

339

340

Table 3 Estimation results of uncorrelated random-parameters Poisson (URPP) model.

Uncorrelated Random-Parameters Poisson (URPP) Model			
Variables	Point Estimate	Standard Error	LRT Statistic
Fixed Parameters			
Log of length [km]	1.33190	0.15999	31.97
Log of percentage of trucks/100	1.54582	0.35773	8.74
SW (Sidewalk: 1 if present; 0 otherwise)	-0.07537	0.07321	0.51
Y2007	-0.21902	0.07875	14.40
Random Parameters			
Constant	2.35492	0.26505	-
<i>standard deviation of parameter distribution: 0.19244</i>			
<i>standard error of standard deviation: 0.02257</i>			
Log of AADT per lane /10,000	2.87971	0.13293	158.63
<i>standard deviation of parameter distribution: 0.58764</i>			
<i>standard error of standard deviation: 0.10062</i>			
LS (Longitudinal slope: 1 if not constant; 0 otherwise)	0.06161	0.05066	7.80
<i>standard deviation of parameter distribution: 0.35332</i>			
<i>standard error of standard deviation: 0.03462</i>			
MV (Mechanical ventilation: 1 if present; 0 otherwise)	0.03655	0.08095	3.07
<i>standard deviation of parameter distribution: 0.31135</i>			
<i>standard error of standard deviation: 0.04945</i>			
Y2008	-0.54294	0.06974	72.75
<i>standard deviation of parameter distribution: 0.08684</i>			
<i>standard error of standard deviation: 0.05154</i>			
Y2009	-0.66468	0.07754	85.39
<i>standard deviation of parameter distribution: 0.26375</i>			
<i>standard error of standard deviation: 0.05746</i>			
Other statistical information			
Number of observations		904	
Log likelihood function $LL(\beta_{URP Model})$		-1449.20704	
Comparison between random-intercept (RIP) and uncorrelated random parameters models (URPP)			
$\chi^2 = -2[LL(\beta_{RIP Model}) - LL(\beta_{URPP Model})]$		10.00	

341

342 One may note that each regression coefficient has the sign coherent with the one corresponding in the
343 aforementioned RIP model; moreover, all variables are statistically significant, except: the sidewalk (SW),
344 and the mechanical ventilation (MV). A reader may see that, in addition to the regression intercept assumed
345 to be a priori random, only 5 parameters were found to be effectively random. These are: AADT, LS, MV,
346 year 2008, and year 2009.347 In particular, AADT per lane is a random parameter that is normally distributed with mean 2.88 and standard
348 deviation 0.588, as a result 99.99% of its distribution is greater than 0. This indicates that all the investigated
349 motorway tunnels present an increase in crash frequency as AADT per lane increases.

350 In a similar way, the longitudinal slope (LS) resulted to be a random parameter that is normally distributed
 351 with mean 0.062 and standard deviation 0.353, as a consequence 57.14% of its distribution is greater than 0
 352 and 42.86% less than 0. This indicates that the majority of tunnels present an increase in crash frequency
 353 when the longitudinal slope is non-constant, but also that a significant minority results in a decrease.
 354 The application of the LRT, between the two competing models (RIP and URPP), shows that the unrelated
 355 random-parameters model is not statistically superior to the corresponding random-intercept model. In fact,
 356 the χ^2 statistic with 5 degrees of freedom is $10.00 < 11.07$. It is to be stressed that this finding might have
 357 been conditioned by the a priori assumption that there is no correlation among random-parameters.

358 *Correlated random-parameters Poisson (CRPP) model*

359 Table 4 shows that the estimation results of the correlated random-parameters Poisson (CRPP) model are
 360 consistent in signs with those of the corresponding unrelated random-parameters Poisson (URPP) model, but
 361 with differences in the magnitude of estimates.
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Table 4 Estimation results of correlated random-parameters Poisson (CRPP) model

Correlated Random-Parameters Poisson (CRPP) Model			
Variables	Point Estimate	Standard Error	LRT Statistic
Fixed Parameters			
Log of length [km]	1.22926	0.16540	28.83
Log of percentage of trucks/100	1.14033	0.37252	4.76
SW (Sidewalk: 1 if present; 0 otherwise)	-0.05781	0.07751	0.26
Y2007	-0.21966	0.08029	14.49
Random Parameters			
Constant	2.07605	0.27454	-
<i>standard deviation of parameter distribution: 0.249881</i>			
Log of AADT per lane /10,000	2.88809	0.14210	155.66
<i>standard deviation of parameter distribution: 0.586767</i>			
LS (Longitudinal slope: 1 if not constant; 0 otherwise)	0.03907	0.05340	10.03
<i>standard deviation of parameter distribution: 0.301248</i>			
MV (Mechanical ventilation: 1 if present; 0 otherwise)	0.01542	0.08817	8.54
<i>standard deviation of parameter distribution: 0.530294</i>			
Y2008	-0.62102	0.07782	95.20
<i>standard deviation of parameter distribution: 0.374315</i>			
Y2009	-0.75360	0.08806	111.07
<i>standard deviation of parameter distribution: 0.444747</i>			
Other statistical information			
Number of observations		904	
Log likelihood function LL(b _{URPP Model})		-1432.88366	
Comparison between uncorrelated (URPP) and correlated random-parameters (CRPP) models			
$\chi^2 = -2[LL(\beta_{URPP Model}) - LL(\beta_{CRPP Model})]$		32.65	
Comparison between random-intercept (RIP) and correlated random-parameters (CRPP) models			
$\chi^2 = -2[LL(\beta_{RIP Model}) - LL(\beta_{CRPP Model})]$		42.65	

364 In the CRPP model all variables are statistically significant including also the mechanical ventilation (MV),
 365 except the sidewalk (SW). The LRT, applied between the two competing models URPP and CRPP, shows
 366 that the correlated random-parameters model is statistically superior to the unrelated random-parameters one.
 367 In particular, the χ^2 statistic with 15 degrees of freedom is $32.65 > 24.99$. This proves that there is a cross
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369 correlation among the random-intercept and the 5 random-parameters (i.e., AADT, LS, MV, year 2008, and
 370 year 2009) that cannot be ignored. In this respect, in our case, the effects of longitudinal slope (*LS*) and
 371 mechanical ventilation (*MV*) are needed to be commented more especially together with annual average daily
 372 traffic (AADT) per lane.

373 For completeness of information, one may also note that the CRPP model is better than the random-intercept
 374 Poisson (RIP) model since the χ^2 with 20 degrees of freedom is $42.65 > 31.41$.

375 Table 5 shows the variance-covariance matrix for the random-parameters (also including the intercept).
 376 Table 6 reports, instead, the correlation coefficient matrix of random parameters. Table 6, in particular,
 377 shows: (i) non-constant longitudinal slope (*LS*) is negatively associated with the annual average daily traffic
 378 (AADT) per lane, this indicates that under non-constant LS conditions the effect of the AADT on increasing
 379 crash frequency could be alleviated; (ii) also mechanical ventilation (*MV*) presents a negative sign with
 380 AADT per lane, which shows that the presence of the mechanical ventilation in tunnels makes less relevant
 381 the influence of AADT per lane on increasing crash frequency.
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Table 5 Variance-covariance matrix of random parameters

Variables	Constant	Log of AADT per lane /10,000	LS	MV	Y2008	Y2009
Constant	0.0624					
Log of AADT per lane /10,000	0.1255	0.3443				
LS (Longitudinal slope: 1 if not constant; 0 otherwise)	0.0181	-0.0480	0.0907			
MV (Mechanical ventilation: 1 if present; 0 otherwise)	-0.1250	-0.2526	-0.0293	0.2812		
Y2008	-0.0519	-0.0264	-0.0851	0.1322	0.1401	
Y2009	0.0561	0.0233	0.0960	-0.1522	-0.1656	0.1978

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Table 6 Correlation coefficient matrix of random parameters

Variables	Constant	Log of AADT per lane /10,000	LS	MV	Y2008	Y2009
Constant	1.00000	0.85601	0.23987	-0.94369	-0.55470	0.50468
Log of AADT per lane /10,000	0.85601	1.00000	-0.27167	-0.81165	-0.12012	0.08915
LS (Longitudinal slope: 1 if not constant; 0 otherwise)	0.23987	-0.27167	1.00000	-0.18365	-0.75502	0.71674
MV (Mechanical ventilation: 1 if present; 0 otherwise)	-0.94369	-0.81165	-0.18365	1.00000	0.66594	-0.64527
Y2008	-0.55470	-0.12012	-0.75502	0.66594	1.00000	-0.99492
Y2009	0.50468	0.08915	0.71674	-0.64527	-0.99492	1.00000

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One possible explanation of these results might be as follows. When the longitudinal slope (*LS*) of tunnel is non-constant, changes in the speed of trucks along the tunnel length are expected. For example, on ascending gradients trucks slow down, while on descending gradients they travel faster. However, since grades of the tunnels investigated are not more than $\pm 4\%$, the effect of non-constant LS on speed of trucks might be limited, and as a result the average speed of trucks might be approximately the same as in flat tunnels. If the longitudinal slope is, instead, constant for example with a positive gradient (upgrade), the speed of trucks continues to decrease along the tunnel length for the grade resistance force that obstacles the truck motion. Negative constant grade also has a significant impact on the speed of trucks, in fact these vehicles in general slow down considerably on downgrades in order to avoid the brakes failure. In the light of the above considerations, the non-constant longitudinal slope (*LS*) of a tunnel has a less effect on the reduction of trucks speed than upgrade or downgrade. As a result, smaller speed differences between trucks and passenger cars might be expected. This means that for maintaining their desirable speed, cars do not have to change lane and overtake trucks more frequently with an increase in traffic flow; as a consequence, for non-constant LS conditions of tunnels, the influence of the AADT per lane on increasing crash frequency could be mitigated.

403 Moreover, when the AADT per lane increases, higher level of pollution due to traffic flow can be expected
404 in tunnels. But if the mechanical ventilation (*MV*) is present, it provides a minimum level of ventilation in
405 order to ensure adequate air quality. This might reduce the negative effects on driving behaviour due, for
406 example, to respiratory defects, eye irritation, as well as to a reduction in visibility in discerning the presence
407 of other vehicles; and as a result a minor influence of AADT per lane on increasing accidents could be
408 expected.

409 On the other hand, we note that the continuous variables enter the model in logarithmic form. Therefore, the
410 estimated coefficient can also be interpreted as the percentage increase in the expected number of accidents
411 corresponding to 1% increase in the variable. For example, from Table 4 one can see that a 1% increase in
412 the AADT per lane produces a 2.888% increase in the expected number of accidents.

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414 **7. Comments for practical applications and discussion for future investigations.**

415 The results of this paper can have potential applications in the design of tunnels. With reference more
416 especially to geometry, road engineers should avoid designing tunnels which are too long. An emergency
417 lane should also be introduced for reducing the probability of accidents. This would also provide an escape
418 lane for vehicles that have broken down and/or emergency services in the case of more relevant events such
419 as accidents followed by fire and/or single burning vehicles caused by electrical-mechanical faults.

420 Generally speaking, the choice of the number of lanes is motivated by the need to relieve traffic congestion.
421 But it is often believed that decreased congestion resulting from a greater number of lanes is also associated
422 with improved safety. This is not always true because by adding another lane, for example passing from two
423 to three lanes, if the traffic flow remains almost the same the opportunities of lane changing would prove to
424 have increased; as a result more traffic conflicts and consequently more accidents might be expected. In this
425 respect, a compromise in the choice of the more appropriate number of lanes would be reached by tunnel
426 designers. With regard to a possible optimal solution, it is to be recorded that specified levels of service
427 (LOS) must be, however, guaranteed in tunnels in relation to entity and composition of traffic flow.

428 It is also the opinion of the authors that the use of safety barriers having a form characterized by a re-
429 directive profile, even if it has not been investigated in this study, should be encouraged in order to reduce
430 the severity of vehicle collisions against the tunnel wall. In addition, the use of porous asphalt in pavement in
431 the transition zones of tunnels (i.e., near the entrance and exit portals), where a higher crash frequency may
432 be expected compared to that of the interior zones, might produce a reduction in accidents caused by adverse
433 weather conditions (e.g., rain, snow, and ice). However, designers should pay attention because the porous
434 asphalt is not advisable in the interior zones of tunnels. In fact, it might not guarantee the drainage in safety
435 conditions of toxic and/or inflammable fluids that might be released by vehicles carrying dangerous goods
436 (DGVs).

437 The predictive model developed can also be used by Tunnel Management Agencies (TMAs) for estimating
438 more accurately variations in accident frequency caused by modifications in the annual average daily traffic
439 (AADT) per lane, in relation to the longitudinal slope (*LS*) of tunnel and mechanical ventilation (*MV*), in
440 order to decide more appropriate safety policies.

441 Although the writers of this paper are confident that they have carried out a suitable analysis, there still
442 remain one point of interest that is worthy of discussion. Tunnel lighting is also a factor that can influence
443 accidents. The quality of lighting, for instance, expressed in terms of density, uniformity, and change in
444 colour, affects the behaviour of drivers within tunnels and consequently their response to traffic and
445 geometry modifications. In our study, the lighting was present in all the tunnels investigated so that the
446 influence of this factor has not been possible to investigate. Since this matter should be dealt with in greater
447 depth, a possible address for future investigations might be an ad-hoc study based on a before-after analysis,
448 in which to compare the count of accidents occurring in tunnels with and without lighting.

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450 **8. Summary and Conclusions**

451 The article explores the use of a correlated random-parameters model to account for the cross correlation
452 among random-parameters, in contrast with the corresponding unrelated random-parameters model and the
453 fixed-parameters one where only the intercept is assumed to vary randomly.

454 The dependent variable is the frequency of total accidents (accidents involving material damage, injuries and
455 fatalities), which occurred in 226 Italian unidirectional motorway tunnels over a four-year monitoring period.
456 The group of dependent variables is: tunnel length (*L*), annual average daily traffic (AADT) per lane,

457 percentage of trucks ($\%Tr$), presence of sidewalk (SW), longitudinal slope (LS), and mechanical ventilation
458 (MV).

459 The so-called random-intercept model (RIP), which was developed a priori for showing the random-effects
460 (temporal correlations among accidents occurring in the same tunnel in different years) confirmed that each
461 variable had the sign expected. The frequency of accidents was positively associated to: L , $AADT$, and $\% Tr$.
462 A reduction in crash frequency over the years was also confirmed, coherently with accident data observed.

463 All variables were found to be statistically significant except: the presence of sidewalk (SW), the longitudinal
464 slope (LS), and the mechanical ventilation (MV).

465 The unrelated random-parameters model (URPP), in which both the intercept and parameters are assumed to
466 vary randomly, showed results consistent with the aforementioned random-intercept model. All variables
467 were found to be statistically significant, except: the sidewalk (SW), and the mechanical ventilation (MV). In
468 addition to the regression intercept (assumed a priori to be random) the following 5 parameters were found to
469 be effectively random: $AADT$, LS , MV , $year\ 2008$, and $year\ 2009$. However, the application of the
470 Likelihood Ratio Test (LRT) showed that the unrelated random-parameters model was not statistically
471 superior to the random-intercept model. This finding might have been conditioned by assuming that there is
472 no correlation among random-parameters.

473 The correlated random-parameters model (CRPP) showed results consistent in signs with those of the
474 corresponding unrelated random-parameters model, but with differences in the magnitude of estimates. All
475 variables were found to be statistically significant except the sidewalk (SW). The LRT proved that the
476 correlated random-parameters model was statistically superior to the corresponding uncorrelated random-
477 parameter and intercept-random models. In other words, a cross correlation exists among the random-
478 intercept and the 5 parameters that were found to be effectively random ($AADT$, LS , MV , $year\ 2008$, and $year$
479 2009). This finding cannot be ignored in statistical analysis. In this respect, in particular, it was found that
480 the non-constant longitudinal slope (LS) of tunnels was negatively associated with the annual average daily
481 traffic ($AADT$) per lane. This indicates that under non-constant LS conditions the effect of the $AADT$ on
482 increasing crash frequency could be mitigated. Also the mechanical ventilation (MV) was found for having a
483 negative sign with $AADT$ per lane. This means that the presence of the mechanical ventilation in tunnels
484 makes less significant the influence of $AADT$ per lane on increasing crash frequency.

485 The correlations found can provide additional insights for future applications to road engineers in the field of
486 tunnel design. Moreover, the model proposed can also be used by Tunnel Management Agencies (TMAs) for
487 the prediction of variations in accident frequency in a specific tunnel attributable to modifications in traffic
488 control systems.

489 Despite the potential of the correlated random-parameters models, it is to be said that the present paper is
490 focused more especially on total accidents. Therefore, a possible direction of expansion of this study could
491 be an analysis based on random-parameters bivariate models in which also severe accidents occurring in
492 tunnels are simultaneously considered. However, it is to be stressed that in this respect a different statistical
493 method should be used. For example, the empirical and/or full Bayes approaches, which are not within the
494 scope of this paper, appear to be more appropriate. In particular, the authors believe that future studies
495 should be addressed towards the development of predictive models for the joint analysis of total and severe
496 accidents occurring in tunnels by applying, for example, the Bayesian random-bivariate method.

497 Another possible extension of this study, which the lack of available information in the data-base has not
498 permitted to be activated in the present paper, should be in making an in-depth investigation of accidents
499 occurring more especially in the transition zones of tunnels, which in combination with different weather and
500 lighting conditions, might occur with greater frequency if compared to those taking place inside tunnels.
501 Also the effectiveness of lighting on the occurrence of accidents in tunnels should be investigated in greater
502 depth, for example by means of an ad-hoc study based on a before-after analysis.

503 Therefore, further research is required for making further development possible.

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